### Memo



To: Anthony Pipolo - Nufusion & Associates

From: Mike Walters - Dillon Consulting Limited

cc: Tim Kooistra - Dillon Consulting Limited

Date: October 13, 2023

Subject: 1460 Lauzon Road Residential Development – TIS Addendum

Our File: 22-3402

1.0

### Introduction

Dillon Consulting Limited (Dillon) has been retained by Nufusion & Associates to develop a transportation impact study (TIS) addendum for the 1460 Lauzon Road residential development in the city of Windsor. This proposed development is located on the southeast corner of the Lauzon Line/McHugh Avenue and Lauzon Road intersection. This addendum was developed to support the original TIS that Dillon prepared in September 2022 and should be reviewed in concert with that TIS.

Compared to the TIS prepared in September 2022, this addendum includes the following items (in response to comments received from City of Windsor staff):

- Provide access recommendations to determine the sufficiency of the right-of-way along Lauzon Road in order to accommodate a sidewalk along the street;
- Determine the need for a land conveyance along Lauzon Road to accommodate a sidewalk on Lauzon Road within the right-of-way;
- Determine the impact on the site buildings using a daylight triangle at the Lauzon Road and McHugh Street intersection;
- Provide revisions to reinforce the right-turn out movement at the McHugh Street driveway;
- Complete a sightline assessment at the McHugh Street driveway to identify potential sightline obstructions for motorists preforming the outbound right-turn movement;
- Based on available traffic data, determine future operations at the Lauzon Road and Spitfire Way intersection; and,
- Complete a parking justification to support the provision of fewer parking spaces on site.

#### 1.1 Proposed Development

The current concept plan (as seen in Appendix A) proposes two (2) 7-storey buildings and one (1) 6-storey building. A total of 291 residential dwellings with 363 vehicle spaces are proposed. The driveway locations and arrangement have remained the same as what was included in the September 2022 TIS.

### 2.0 Access Recommendations

The McHugh Street driveway is proposed to be channelized and would only permit the right-out movement, while the Lauzon Road driveway is a full-movement driveway.

Dillon confirmed the sufficiency of the existing right-of-way to accommodate a potential sidewalk along Lauzon Road. The required driveway arrangements are summarized in Table 1 and are provided in Appendix A.

Table 1: Driveway Requirements

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Driveway	Requirements
	4.5 metres wide channelized driveway
McHugh Street	15.5 metre outside radius
	11.0 metre inside radius
Lauzan Daad	9.0 metres wide driveway behind the sidewalk (in accordance with City standard)
Lauzon Road	2.0 metre driveway flare between road edge and sidewalk (in accordance with City standard)

## Daylight Triangle Assessment

As per the City of Windsor's bylaw NO. 250-2004, a 6 metre by 6 metre daylight triangle is required on the southeast corner of the McHugh Street/Lauzon Line and Lauzon Road intersection. A 6 metre by 6 metre daylight triangle is now shown on the current concept plan (located in Appendix A). No obstructions will be present within the daylight triangle.

### Sightline Assessment

A sightline assessment was undertaken for the proposed McHugh Street right-out driveway. This assessment referenced requirements outlined in TAC's *Geometric Design Guide for Canadian Roads* (June 2017).

Table 2 outlines the required sight distance for the proposed right-out movement at the McHugh Street driveway.

**Table 2: TAC Intersection Sight Distance** 

Design Speed	Case	Intersection Sight Distance
60 km/h	Case B2 – Right Turn from Minor Road	110 metres

The driver decision point was located 4.4 metres back from the south curb on McHugh Street. The required sight distance is measured from the decision point west along the centre of the eastbound approach lane, until the measurement reaches 110 metres. The measurement along the centre of the travel lane is not along a straight line as it follows the horizontal curve on McHugh Street.

Currently, McHugh Street does not have a raised median and grades are 3% or less. Therefore, no time gap adjustments were applied to either lengthen or shorten the required sight distance.

Figure 1 displays the sightline area for a vehicle exiting the McHugh Street driveway.

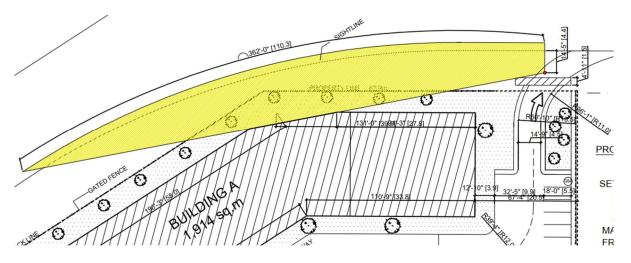


Figure 1: Sightline Assessment Results

Based on the sightline assessment, a portion of Building 'A' would have originally blocked a driver's sightline when turning right at the McHugh Street driveway. As a result, a portion of Building 'A' was modified so that the building is outside the identified sightline area.

### **Intersection Operations**

#### Traffic Volumes

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Turning movement count (TMC) and traffic signal timing data was provided by the City of Windsor for the Lauzon Road and Spitfire Way intersection. The raw traffic data can be found in Appendix B.

This TMC was not conducted during a Saturday; therefore, the existing Saturday peak hour volumes were estimated. For the analysis, the existing Saturday inbound and outbound volumes to/from Spitfire Way were assumed to be the same as what was collected during the weekday PM peak hour. The Saturday northbound and southbound through movements were estimated by relying on traffic data previously estimated along Lauzon Road to the north of Spitfire Way.

The 2022 traffic data was adjusted to be representative of 2023 data by applying a 1.0% per annum compounded growth rate to all movements at the intersection.

#### 5.1.1 Existing (2023) Traffic Volumes

Figure 2 shows the existing (2023) traffic volumes at the Lauzon Road and Spitfire Way intersection.

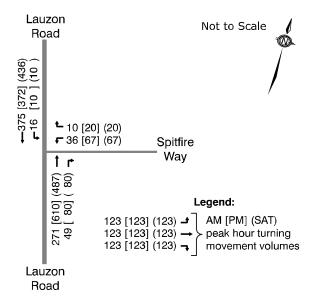


Figure 2: Existing (2023) Traffic Volumes

#### 5.1.2 Background Development Traffic Volumes

The same four background developments that were discussed in the original TIS were included in this addendum's future background analysis. It should be noted that some of these developments are currently in the process of being built, and some of the existing traffic volumes collected at the Lauzon Road and Spitfire Way intersection may include some trips generated by these background developments. However, in order to allow the subsequent findings to be more conservative, no adjustments to the background development traffic volumes were made. Figure 3 and Figure 4 summarize the volumes generated by four various background developments in both 2025 and 2030.

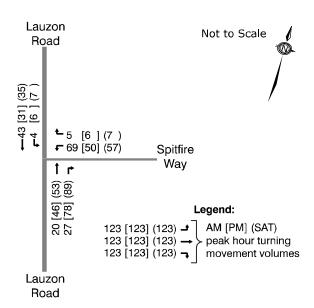


Figure 3: 2025 Background Development Traffic Volumes

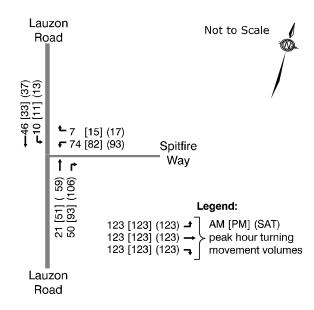


Figure 4: 2030 Background Development Traffic Volumes

#### *5.1.3* Future Background Traffic Volumes

Figure 5 and Figure 6 show the future background volumes in 2025 and 2030, respectively, at the Lauzon Road and Spitfire Way intersection.

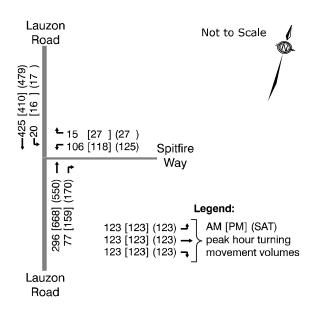


Figure 5: Future (2025) Background Traffic Volumes

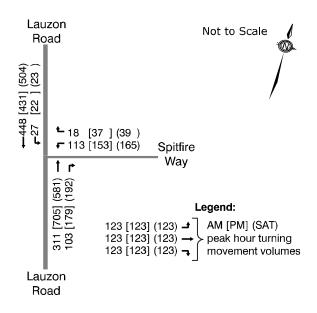


Figure 6: Future (2030) Background Traffic Volumes

#### 5.1.4 Trip Generation

Table 3 summarizes the number of vehicle trips anticipated to be generated by the proposed residential development, noting that 291 dwelling units are proposed.

**Table 3: Trip Generation** 

Dovolonment	ΙA	M peak ho	our	PI	M peak ho	ur	Satu	rday peak	hour
Development	In	Out	Total	In	Out	Total	In	Out	Total
SITE-GENERATED TRIPS	<u> </u>								
Apartments (ITE Land I	Jse Code 2	?21)							
% in/out, trip generation rate	23%	77%	0.37	61%	39%	0.39	51%	49%	0.39
Site trips (291 units)	25	83	108	69	44	113	58	55	113
TOTAL GROSS SITE TRIPS	25	83	108	69	44	113	58	55	113

The proposed residential development at 1460 Lauzon Road is now forecast to generate 108 vehicle trips in the weekday AM peak hour (25 inbound, 83 outbound), 113 vehicle trips in the weekday PM peak hour (69 inbound, 44 outbound), and 113 vehicle trips during the Saturday mid-day peak hour (58 inbound, 55 outbound).

These trips were assigned and distributed in the same manner as the originally submitted TIS. Figure 7 shows how these site-generated trips were assigned through the Lauzon Road and Spitfire Way intersection.

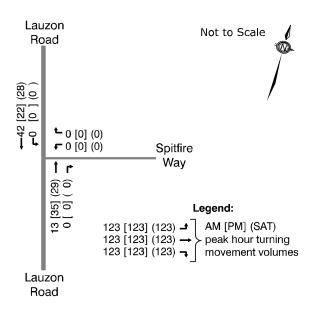


Figure 7: Site-Generated Trips

#### 5.1.5 Total Future Traffic Volumes

Figure 8 and Figure 9 show the total future volumes in 2025 and 2030 at the Lauzon Road and Spitfire Way intersection.

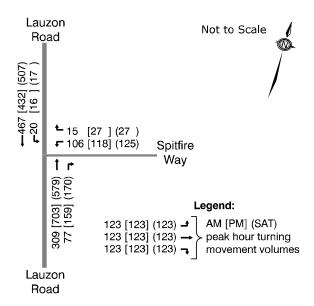


Figure 8: Total Future (2025) Traffic Volumes

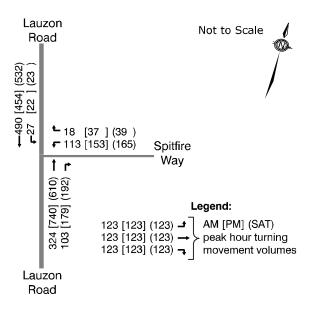


Figure 9: Total Future (2030) Traffic Volumes

### **Lauzon Road and Spitfire Way - Intersection Operations**

5.2

Intersection operations at the Lauzon Road and Spitfire Way signalized intersection were determined based on the methodology outlined in the *Highway Capacity Manual* (HCM) and facilitated using Synchro analysis software. The signalized intersection analyses are based on existing lane configurations, traffic signal phasing and timings (as provided by the City of Windsor) present during the AM, PM and Saturday peak hours. Currently, the Lauzon Road and Spitfire Way intersection operates in

an actuated coordinated manner with a 50-second cycle length in the AM peak hour, and a 53-second cycle length in the PM peak hour and the Saturday mid-day peak hour.

The overall level of service<sup>1</sup> and average vehicle delay have been noted. Also, for each movement, the volume-to-capacity ratio, level of service, average delay and 95<sup>th</sup> percentile queue have been identified. The Synchro analysis worksheets are provided in Appendix C. The results were reviewed to identify any critical movements, defined as follows:

- Any through lane/movement with a v/c ratio of 0.85 or higher;
- Any exclusive turning lane/movement with a v/c ratio of 1.00 or higher;
- Any movement at a signalized intersection operating at LOS F; or
- Any turning movement with a 95<sup>th</sup> percentile queue exceeding the available storage.

Table 4 summarizes the operations at the Lauzon Road and Spitfire Way intersection.

**Table 4: Lauzon Road and Spitfire Way Signalized Intersection Operations** 

	١	Neekda	ay AM Pea	ak Hour		Weekda	ay PM Pea	ak Hour	Sat	urday N	√lid-Day p	eak hour
Movement	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>
Existing (2023)	) Signal	ized Int	ersection	Operations								
WBL	0.14	В	15.7	7	0.21	В	18.0	12	0.21	В	18.0	12
WBR	0.04	Α	7.6	2	0.06	Α	7.5	4	0.06	Α	7.5	4
NBTR	0.16	Α	4.3	15	0.26	Α	4.6	33	0.22	Α	4.4	26
SBTL	0.21	Α	4.9	20	0.15	Α	4.5	19	0.18	Α	4.6	22
Overall	_	Α	5.2	_	_	Α	5.4	_	_	Α	5.3	_
Future Backgr	ound (2	2025) Si	gnalized l	ntersection O	peratio	ons						
WBL	0.38	В	18.8	16	0.34	В	19.4	18	0.36	В	19.6	19
WBR	0.06	Α	6.9	3	0.08	Α	6.8	4	0.08	Α	6.7	4
NBTR	0.21	Α	4.9	16	0.36	Α	5.7	39	0.31	Α	5.3	32
SBTL	0.27	Α	6.0	23	0.20	Α	5.5	21	0.23	Α	5.7	25
Overall	_	Α	7.1	_	_	Α	6.8	_	_	Α	6.7	_
Total Future (2	2025) S	ignalize	d Intersed	ction Operatio	ons							
WBL	0.38	В	18.8	16	0.34	В	19.4	18	0.36	В	19.6	19
WBR	0.06	Α	6.9	3	0.08	Α	6.8	4	0.08	Α	6.7	4
NBTR	0.22	Α	5.0	17	0.37	Α	5.9	42	0.32	Α	5.4	34
SBTL	0.29	Α	6.2	25	0.21	Α	5.5	22	0.24	Α	5.7	26
Overall	_	Α	7.1	_	_	Α	6.9	_	_	Α	6.8	_

<sup>&</sup>lt;sup>1</sup> Level of Service (LOS), applied to an intersection, is a measure qualifying the amount of delay experienced by motorists, expressed either for specific turning movements or for the intersection as a whole. A more detailed explanation of LOS is provided in *Appendix D*.

	١	Weekda	ay AM Pea	ak Hour	,	Weekda	ay PM Pea	ak Hour	Sat	urday N	Mid-Day p	eak hour
Movement	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>	v/c	LOS	Delay (s/veh)	95 <sup>th</sup> %ile queue <i>(m)</i>
Future Backgr	ound (2	2030) Si	gnalized I	ntersection O	peration	ons						
WBL	0.40	В	19.0	16	0.42	С	20.2	23	0.44	С	20.5	25
WBR	0.07	Α	6.5	3	0.10	Α	6.1	5	0.10	Α	6.0	5
NBTR	0.24	Α	4.8	17	0.39	Α	6.2	43	0.34	Α	5.7	34
SBTL	0.29	Α	6.3	25	0.22	Α	5.9	22	0.25	Α	6.1	26
Overall	_	Α	7.1	_	_	Α	7.5	_	_	Α	7.5	_
Total Future (2	2030) S	ignalize	d Intersed	ction Operatio	ons							
WBL	0.40	В	19.0	16	0.42	С	20.2	23	0.44	С	20.5	25
WBR	0.07	Α	6.5	3	0.10	Α	6.1	5	0.10	Α	6.0	5
NBTR	0.25	Α	4.9	18	0.40	Α	6.4	45	0.35	Α	5.9	36
SBTL	0.32	Α	6.4	27	0.23	Α	5.9	24	0.26	Α	6.2	28
Overall	_	Α	7.2	_	_	Α	7.6	_	_	Α	7.5	_

The Lauzon Road and Spitfire Way intersection is projected to operate at an acceptable level of service through to the 2030 horizon, both with and without the subject residential development in place. During the three peak hours, all movements are projected to operate at LOS C or better with minimal queues and delays projected. In addition, the intersection is projected to continue operating at LOS A overall during the weekday AM, PM and Saturday mid-day peak hours.

### 6.0 Parking Requirements

As seen in Appendix A, the updated concept plan now proposes 363 parking spaces. Based on the proposed residential land use, the Windsor Zoning By-Law requires 1.25 parking spaces per dwelling unit. Since 291 dwelling units are proposed, 363 parking spaces (291 X 1.25 = 363) are required. As the number of parking spaces now equals the By-Law requirement, no parking justification study is required.

## 7.0 Summary

Dillon Consulting Limited (Dillon) has been retained by Nufusion & Associates to prepare a TIS addendum for the proposed residential development at 1460 Lauzon Road in the city of Windsor. This addendum was developed to support the original TIS prepared in September 2022.

Based on the sightline assessment, a portion of Building 'A' would have originally blocked a driver's view when turning right at the McHugh Street driveway. As a result, a portion of Building 'A' was modified so no portion of the building is located within the identified sightline.

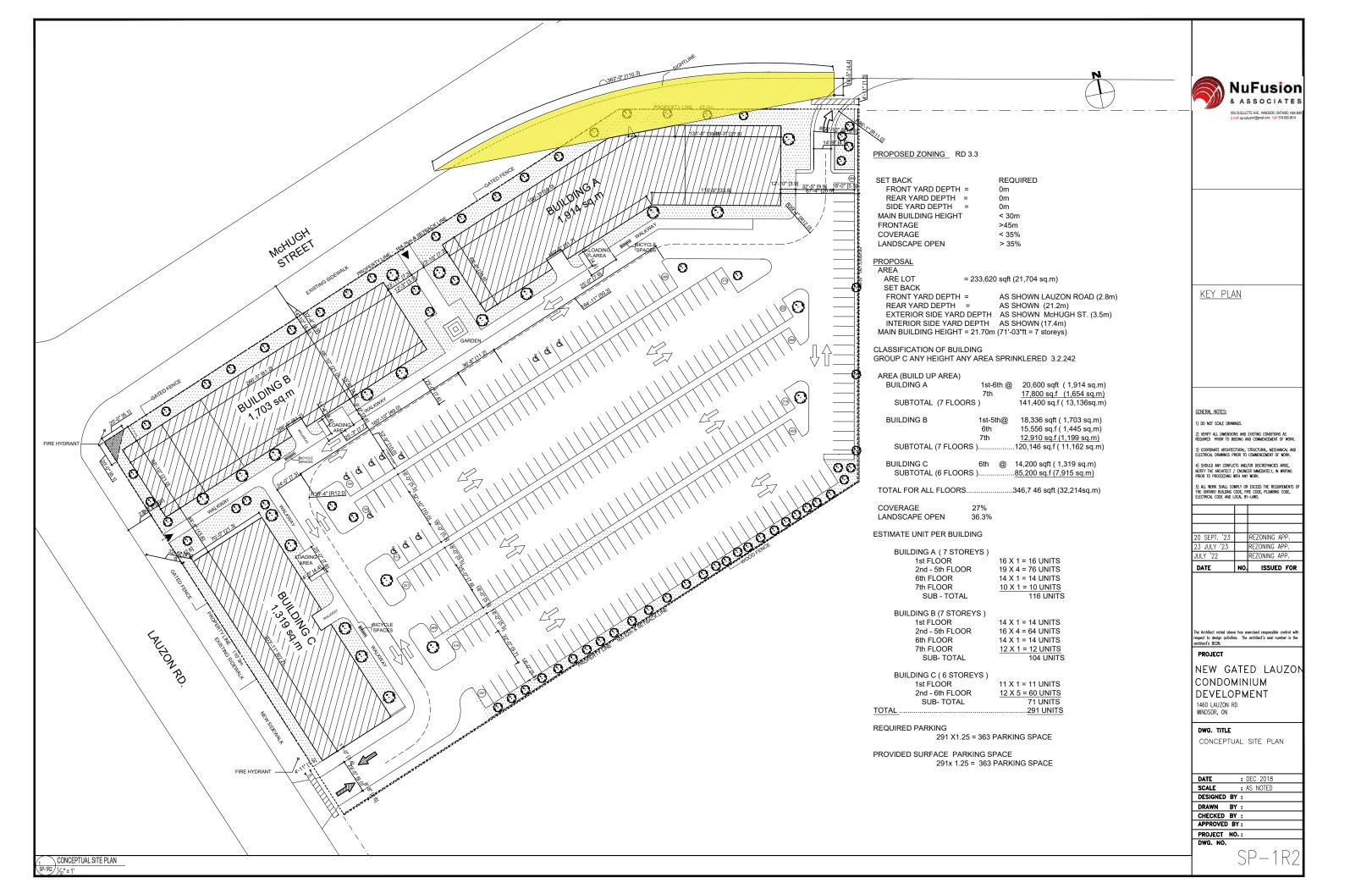
The Lauzon Road and Spitfire Way signalized intersection will operate in an acceptable manner through to the 2030 horizon both with and without the subject residential development in place. During the three time periods, all movements are projected to operate at LOS C or better with minimal queuing and delays. In addition, the intersection will continue to operate at LOS A overall A through to the 2030 horizon year.

Since the site now proposes to provide the required number of parking spaces according to the City's Zoning By-Law, a parking justification study is no longer required.



# Appendix A

Concept Plan



## Appendix B

**Turning Movement Count Data** 



### **Project #22-074 - City of Windsor**

### **Intersection Count Report**

**Intersection:** LAUZON ROAD & SPITFIRES WAY

Municipality: Windsor

**Count Date:** Wednesday, Sep 14, 2022

**Site Code:** 2207400001

**Count Categories:** Cars, Medium Trucks, Heavy Trucks, Peds, Bicycles

**Count Period:** 07:00-10:00, 11:00-14:00, 15:00-18:00

**Weather:** Clear

**Comments:** 

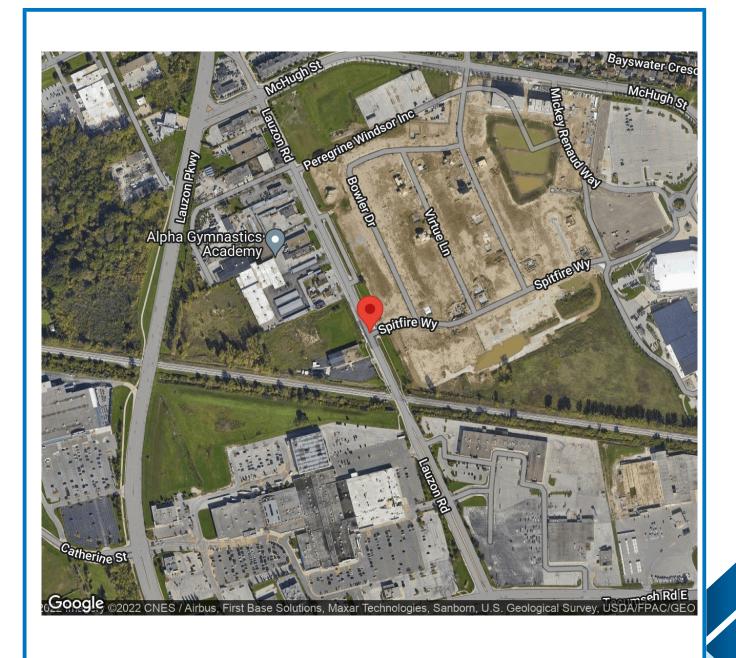


### **Traffic Count Map**

Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001 Municipality: Windsor

Count Date: Sep 14, 2022





### **Traffic Count Summary**

Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **LAUZON ROAD - Traffic Summary**

		North	Appr	oach T	otals			South	Appro	oach T	otals		
	Includ	les Cars,	Medium Bicy	Trucks, I cles	Heavy Tru	ıcks,	Includ	les Cars, l	Medium Bicy		Heavy Tru	ıcks,	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	14	316	0	0	330	1	0	250	60	0	310	2	640
08:00 - 09:00	15	316	0	0	331	0	0	267	33	0	300	1	631
09:00 - 10:00	22	328	0	0	350	1	0	282	30	0	312	0	662
						BREAK						_	
11:00 - 12:00	17	348	0	0	365	0	0	373	44	0	417	4	782
12:00 - 13:00	11	348	0	0	359	0	0	421	56	0	477	2	836
13:00 - 14:00	9	310	0	0	319	5	0	407	52	0	459	1	778
					I	BREAK							
15:00 - 16:00	11	362	0	0	373	1	0	483	68	0	551	7	924
16:00 - 17:00	10	368	0	0	378	0	0	604	79	0	683	5	1061
17:00 - 18:00	10	302	0	0	312	0	0	627	82	1	710	6	1022
GRAND TOTAL	119	2998	0	0	3117	8	0	3714	504	1	4219	28	7336



### **Traffic Count Summary**

Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **SPITFIRES WAY - Traffic Summary**

		East	Appro	ach To	tals			West	Appro	ach To	tals		
	Includ	les Cars, I	Medium Bicy	Trucks, H cles	leavy Tri	ucks,	Includ	es Cars, I	Medium Bicy	Trucks, H cles	leavy Tru	ıcks,	
Hour	Left	Thru	Right	U-Turn	Total	Peds	Left	Thru	Right	U-Turn	Total	Peds	Total
07:00 - 08:00	47	0	9	0	56	2	0	0	0	0	0	0	56
08:00 - 09:00	15	0	6	0	21	1	0	0	0	0	0	0	21
09:00 - 10:00	32	0	19	0	51	1	0	0	0	0	0	0	51
						BREAK .							
11:00 - 12:00	62	0	12	0	74	2	0	0	0	0	0	0	74
12:00 - 13:00	39	0	6	0	45	0	0	0	0	0	0	0	45
13:00 - 14:00	38	0	7	0	45	3	0	0	0	0	0	0	45
						BREAK							
15:00 - 16:00	71	0	7	0	78	4	0	0	0	0	0	0	78
16:00 - 17:00	66	0	20	0	86	0	0	0	0	0	0	0	86
17:00 - 18:00	49	0	5	0	54	4	0	0	0	0	0	0	54
GRAND TOTAL	419	0	91	0	510	17	0	0	0	0	0	0	510



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### North Approach - LAUZON ROAD

			Cars				Mediu	ım Tru	cks			Heav	y Trucl	ks			В	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
07:00	5	52	0	0	57	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	1
07:15	1	56	0	0	57	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	0
07:30	5	74	0	0	79	0	1	0	0	1	0	3	0	0	3	0	0	0	0	0	0
07:45	3	121	0	0	124	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0
08:00	2	84	0	0	86	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
08:15	5	78	0	0	83	0	0	0	0	0	1	3	0	0	4	0	0	0	0	0	0
08:30	3	65	0	0	68	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	0
08:45	2	75	0	0	77	1	0	0	0	1	1	3	0	0	4	0	0	0	0	0	0
09:00	3	73	0	0	76	0	2	0	0	2	0	1	0	0	1	0	0	0	0	0	0
09:15	6	82	0	0	88	1	1	0	0	2	1	3	0	0	4	0	0	0	0	0	1
09:30	4	79	0	0	83	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0
09:45	7	83	0	0	90	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	46	922	0	0	968	2	12	0	0	14	3	25	0	0	28	0	1	0	0	1	2



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### North Approach - LAUZON ROAD

			Cars				Mediu	ım Tru	cks			Heav	vy Truc	ks			В	icycles			T. 4.1
Start Time	4	1	•	1	Total	4	1	-	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	3	76	0	0	79	0	0	0	0	0	1	2	0	0	3	1	0	0	0	1	0
11:15	1	93	0	0	94	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
11:30	6	74	0	0	80	0	2	0	0	2	0	1	0	0	1	0	0	0	0	0	0
11:45	4	96	0	0	100	0	0	0	0	0	1	2	0	0	3	0	0	0	0	0	0
12:00	2	73	0	0	75	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0
12:15	2	76	0	0	78	0	3	0	0	3	1	1	0	0	2	0	0	0	0	0	0
12:30	2	89	0	0	91	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
12:45	4	96	0	0	100	0	3	0	0	3	0	2	0	0	2	0	0	0	0	0	0
13:00	2	81	0	0	83	0	1	0	0	1	1	0	0	0	1	0	0	0	0	0	2
13:15	2	76	0	0	78	0	1	0	0	1	0	2	0	0	2	0	0	0	0	0	1
13:30	2	72	0	0	74	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
13:45	1	72	0	0	73	0	2	0	0	2	0	3	0	0	3	0	0	0	0	0	1
SUBTOTAL	31	974	0	0	1005	0	14	0	0	14	5	18	0	0	23	1	0	0	0	1	5



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001 Municipality: Windsor

Count Date: Sep 14, 2022

### North Approach - LAUZON ROAD

			Cars				Medi	um Tru	cks			Heav	vy Truc	ks			Bi	icycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	4	1	•	1	Total	Total Peds
15:00	4	97	0	0	101	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	(
15:15	1	91	0	0	92	0	0	0	0	0	1	1	0	0	2	0	1	0	0	1	(
15:30	1	73	0	0	74	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
15:45	3	92	0	0	95	0	2	0	0	2	1	1	0	0	2	0	1	0	0	1	
16:00	2	100	0	0	102	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
16:15	2	83	0	0	85	0	1	0	0	1	1	1	0	0	2	0	1	0	0	1	
16:30	2	89	0	0	91	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
16:45	3	93	0	0	96	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:00	1	81	0	0	82	0	1	0	0	1	1	0	0	0	1	0	0	0	0	0	
17:15	2	83	0	0	85	0	1	0	0	1	1	0	0	0	1	0	0	0	0	0	
17:30	4	68	0	0	72	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:45	1	68	0	0	69	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	(
SUBTOTAL	26	1018	0	0	1044	0	8	0	0	8	5	3	0	0	8	0	3	0	0	3	
GRAND TOTAL	103	2914	0	0	3017	2	34	0	0	36	13	46	0	0	59	1	4	0	0	5	:



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **South Approach - LAUZON ROAD**

			Cars				Mediu	m Tru	cks			Heav	y Trucl	(S			Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	-	1	Total	Total Peds
07:00	0	37	8	0	45	0	0	0	0	0	0	2	1	0	3	0	0	0	0	0	0
07:15	0	46	20	0	66	0	2	0	0	2	0	4	0	0	4	0	0	0	0	0	1
07:30	0	71	18	0	89	0	0	0	0	0	0	5	0	0	5	0	1	0	0	1	1
07:45	0	81	13	0	94	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0
08:00	0	41	10	0	51	0	0	1	0	1	0	3	1	0	4	0	0	0	0	0	0
08:15	0	63	6	0	69	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
08:30	0	75	5	0	80	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0
08:45	0	79	10	0	89	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1
09:00	0	64	8	0	72	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0
09:15	0	74	7	0	81	0	0	0	0	0	0	2	0	0	2	0	0	1	0	1	0
09:30	0	64	9	0	73	0	1	0	0	1	0	2	0	0	2	0	0	0	0	0	0
09:45	0	71	5	0	76	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
SUBTOTAL	0	766	119	0	885	0	5	1	0	6	0	27	2	0	29	0	1	1	0	2	3



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **South Approach - LAUZON ROAD**

			Cars				Mediu	m Tru	cks			Heav	y Trucl	ks			Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	0	77	12	0	89	0	1	1	0	2	0	1	0	0	1	0	0	0	0	0	1
11:15	0	85	7	0	92	0	1	1	0	2	0	4	0	0	4	0	0	0	0	0	0
11:30	0	98	4	0	102	0	2	1	0	3	0	1	0	0	1	0	1	1	0	2	1
11:45	0	98	15	0	113	0	1	0	0	1	0	3	0	0	3	0	0	2	0	2	2
12:00	0	104	12	0	116	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
12:15	0	101	14	0	115	0	3	1	0	4	0	2	0	0	2	0	0	0	0	0	1
12:30	0	105	17	0	122	0	2	1	0	3	0	0	0	0	0	0	0	0	0	0	0
12:45	0	98	11	0	109	0	3	0	0	3	0	2	0	0	2	0	0	0	0	0	1
13:00	0	97	16	0	113	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:15	0	88	10	0	98	0	0	1	0	1	0	1	1	0	2	0	0	0	0	0	0
13:30	0	99	13	0	112	0	1	1	0	2	0	2	0	0	2	0	0	0	0	0	1
13:45	0	115	10	0	125	0	1	0	0	1	0	3	0	0	3	0	0	0	0	0	0
SUBTOTAL	0	1165	141	0	1306	0	16	7	0	23	0	19	1	0	20	0	1	3	0	4	7



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **South Approach - LAUZON ROAD**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks			Bi	cycles			Total
Start Time	- 4	1	•	J.	Total	4	1	•	J.	Total	4	1	•	1	Total	4	1	•	J	Total	Total Peds
15:00	0	114	9	0	123	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0	0
15:15	0	132	18	0	150	0	1	0	0	1	0	1	0	0	1	0	0	1	0	1	0
15:30	0	107	21	0	128	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	2
15:45	0	125	18	0	143	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	5
16:00	0	150	14	0	164	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15	0	149	22	0	171	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0
16:30	0	157	18	0	175	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	2
16:45	0	145	25	0	170	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
17:00	0	143	21	1	165	0	0	0	0	0	0	0	1	0	1	0	1	0	0	1	4
17:15	0	182	25	0	207	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:30	0	152	21	0	173	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	2
17:45	0	148	13	0	161	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	0	1704	225	1	1930	0	6	1	0	7	0	3	1	0	4	0	1	2	0	3	18
GRAND TOTAL	0	3635	485	1	4121	0	27	9	0	36	0	49	4	0	53	0	3	6	0	9	28



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001 Municipality: Windsor

Count Date: Sep 14, 2022

### **East Approach - SPITFIRES WAY**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks			Bi	cycles			Total
Start Time	4	1	-	J.	Total	4	1	•	J	Total	4	1	•	J.	Total	4	1	•	J.	Total	Total Peds
07:00	4	0	1	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15	10	0	0	0	10	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0
07:30	16	0	3	0	19	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
07:45	13	0	5	0	18	1	0	0	0	1	0	0	0	0	0	1	0	0	0	1	1
08:00	1	0	0	0	1	0	0	0	0	0	1	0	1	0	2	0	0	0	0	0	1
08:15	1	0	1	0	2	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
08:30	5	0	2	0	7	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0
08:45	4	0	2	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00	5	0	4	0	9	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	1
09:15	5	0	3	0	8	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
09:30	10	0	4	0	14	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
09:45	8	0	8	0	16	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
SUBTOTAL	82	0	33	0	115	2	0	0	0	2	7	0	1	0	8	3	0	0	0	3	4



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001

Municipality: Windsor

Count Date: Sep 14, 2022

### **East Approach - SPITFIRES WAY**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks			Bi	cycles			Takal
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
11:00	5	0	4	0	9	0	0	1	0	1	1	0	0	0	1	3	0	0	0	3	1
11:15	11	0	1	0	12	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
11:30	22	0	3	0	25	1	0	0	0	1	1	0	0	0	1	1	0	0	0	1	0
11:45	16	0	3	0	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
12:00	12	0	2	0	14	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
12:15	10	0	1	0	11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30	6	0	2	0	8	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0
12:45	6	0	1	0	7	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0
13:00	16	0	2	0	18	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
13:15	4	0	3	0	7	0	0	1	0	1	1	0	0	0	1	0	0	0	0	0	1
13:30	7	0	0	0	7	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
13:45	7	0	1	0	8	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0	0
SUBTOTAL	122	0	23	0	145	6	0	2	0	8	7	0	0	0	7	4	0	0	0	4	5



Intersection: LAUZON ROAD & SPITFIRES WAY

Site Code: 2207400001 Municipality: Windsor

Count Date: Sep 14, 2022

### **East Approach - SPITFIRES WAY**

			Cars				Mediu	ım Tru	cks			Heav	y Truc	ks			Bi	cycles			Total
Start Time	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	4	1	•	1	Total	Total Peds
15:00	22	0	1	0	23	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
15:15	19	0	1	0	20	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
15:30	10	0	2	0	12	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2
15:45	17	0	3	0	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
16:00	23	0	8	0	31	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0
16:15	13	0	6	0	19	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
16:30	16	0	5	0	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:45	11	0	1	0	12	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0
17:00	15	0	2	0	17	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
17:15	15	0	1	0	16	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1
17:30	12	0	2	0	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
17:45	5	0	0	0	5	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
SUBTOTAL	178	0	32	0	210	2	0	0	0	2	5	0	0	0	5	1	0	0	0	1	8
GRAND TOTAL	382	0	88	0	470	10	0	2	0	12	19	0	1	0	20	8	0	0	0	8	17



### **Peak Hour Diagram**

**Specified Period** 

**One Hour Peak** 

From: To: 07:00:00 10:00:00

From: 07:30:00

To:

08:30:00

Intersection: LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

Weather conditions:

Clear

#### \*\* Unsignalized Intersection \*\*

#### Major Road: LAUZON ROAD runs N/S

#### **North Approach**

	Out	In	Total
	372	265	637
MT	3	1	4
HT	12	11	23
₫ <b>%</b>	0	1	1
	387	278	665

#### **LAUZON ROAD**

		<b>1</b>	LÎ.
Totals	371	16	0
	357	15	0
MT	3	0	0
HT	11	1	0
<i>₫</i>	0	0	0

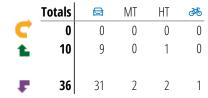
#### **East Approach**

	Out	In	Total
	40	62	102
MT	2	1	3
ΗТ	3	2	5
₹6	1	0	1
	46	65	111

#### Peds: 0



#### **SPITFIRES WAY**



Peds: 1

	<b>†</b>		J
Totals	268	49	0
	256	47	0
MT	1	1	0
HT	10	1	0
<i>₫</i>	1	0	0

**LAUZON ROAD** 

#### **South Approach**

	Out	In	Total
	303	388	691
MT	2	5	7
НТ	11	13	24
<b>ॐ</b>	1	1	2
	317	407	724



MT - Medium Trucks

HT - Heavy Trucks

♣ - Bicycles

#### **Comments**



### **Peak Hour Summary**

Intersection: LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

 Period:
 07:00 - 10:00

### **Peak Hour Data (07:30 - 08:30)**

		N	lorth A LAUZO	pproac N ROAE	:h )			S	outh A LAUZO	pproac N ROAD	h )				East Ap SPITFIR	pproacl RES WA	h Y		West Approach				Total Vehicl		
Start Time	•	1	•	J	Peds	Total	4	t	•	•	Peds	Total	4	1	•	•	Peds	Total	4	1	•	O	Peds	Total	es
07:30	5	78		0	0	83		77	18	0	1	95	17		3	0	0	20					0		198
07:45	3	124		0	0	127		82	13	0	0	95	15		5	0	1	20					0		242
08:00	2	88		0	0	90		44	12	0	0	56	2		1	0	1	3					0		149
08:15	6	81		0	0	87		65	6	0	0	71	2		1	0	0	3					0		161
Grand Total	16	371		0	0	387		268	49	0	1	317	36		10	0	2	46					0	0	750
Approach %	4.1	95.9		0		-		84.5	15.5	0		-	78.3		21.7	0		-						-	
Totals %	2.1	49.5		0		51.6		35.7	6.5	0		42.3	4.8		1.3	0		6.1						0	
PHF	0.67	0.75		0		0.76		0.82	0.68	0		0.83	0.53		0.5	0		0.58						0	0.77
Cars	15	357		0		372		256	47	0		303	31		9	0		40						0	715
% Cars	93.8	96.2		0		96.1		95.5	95.9	0		95.6	86.1		90	0		87						0	95.3
Medium Trucks	0	3		0		3		1	1	0		2	2		0	0		2						0	7
% Medium Trucks	0	0.8		0		0.8		0.4	2	0		0.6	5.6		0	0		4.3						0	0.9
Heavy Trucks	1	11		0		12		10	1	0		11	2		1	0		3						0	26
% Heavy Trucks	6.3	3		0		3.1		3.7	2	0		3.5	5.6		10	0		6.5						0	3.5
Bicycles	0	0		0		0		1	0	0		1	1		0	0		1						0	2
% Bicycles	0	0		0		0		0.4	0	0		0.3	2.8		0	0		2.2						0	0.3
Peds % Peds					0	-					1 33.3	-					2 66.7	-					0	-	3



### **Peak Hour Diagram**

#### **Specified Period**

#### **One Hour Peak**

From: 11:00:00 To: 14:00:00 From: 11:45:00 To: 12:45:00

Intersection: LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

Weather conditions:

Clear

#### \*\* Unsignalized Intersection \*\*

#### Major Road: LAUZON ROAD runs N/S

#### **North Approach**

	Out	In	Total
	344	416	760
MT	4	7	11
HT	9	5	14
₫ <b>%</b>	0	0	0
	357	428	785

#### **LAUZON ROAD**

		<b>1</b>	. 1
Totals	345	12	0
	334	10	0
MT	4	0	0
HT	7	2	0
<i>₫</i>	0	0	0

#### **East Approach**

	Out	In	Total
	52	68	120
MT	2	2	4
HT	1	2	3
<b>ॐ</b>	0	2	2
	55	74	129

#### Peds: 0



### SPITFIRES WAY

	Totals		MT	HT	<i>₫</i>
C	0	0	0	0	0
£	8	8	0	0	0
F	47	44	2	1	0

Peds: 3

	1	•	J.
Totals	420	62	0
	408	58	0
MT	7	2	0
HT	5	0	0
<i>₫</i>	0	2	0

#### **LAUZON ROAD**

#### South Approach

	Out	In	Total
	466	378	844
MT	9	6	15
HT	5	8	13
<b>ॐ</b>	2	0	2
	482	392	874

📾 - Cars

MT - Medium Trucks

HT - Heavy Trucks

♣ - Bicycles

#### **Comments**



### **Peak Hour Summary**

Intersection: LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

 Period:
 11:00 - 14:00

### **Peak Hour Data (11:45 - 12:45)**

				ipproac N ROAL				S	outh A	pproac N ROAE	h )				East Ap SPITFIR						West A	pproacl	1		Total Vehicl
Start Time	•	1	•	•	Peds	Total	•	1	•	•	Peds	Total	4	1	•	•	Peds	Total	4	1	•	•	Peds	Total	es
11:45	5	98		0	0	103		102	17	0	2	119	16		3	0	1	19					0		241
12:00	2	76		0	0	78		105	12	0	0	117	13		2	0	0	15					0		210
12:15	3	80		0	0	83		106	15	0	1	121	10		1	0	0	11					0		215
12:30	2	91		0	0	93		107	18	0	0	125	8		2	0	0	10					0		228
Grand Total	12	345		0	0	357		420	62	0	3	482	47		8	0	1	55					0	0	894
Approach %	3.4	96.6		0		1		87.1	12.9	0		-	85.5		14.5	0		-						-	
Totals %	1.3	38.6		0		39.9		47	6.9	0		53.9	5.3		0.9	0		6.2						0	
PHF	0.6	0.88		0		0.87		0.98	0.86	0		0.96	0.73		0.67	0		0.72						0	0.93
Cars	10	334		0		344		408	58	0		466	44		8	0		52						0	862
% Cars	83.3	96.8		0		96.4		97.1	93.5	0		96.7	93.6		100	0		94.5						0	96.4
Medium Trucks	0	4		0		4		7	2	0		9	2		0	0		2						0	15
% Medium Trucks	0	1.2		0		1.1		1.7	3.2	0		1.9	4.3		0	0		3.6						0	1.7
Heavy Trucks	2	7		0		9		5	0	0		5	1		0	0		1						0	15
% Heavy Trucks	16.7	2		0		2.5		1.2	0	0		1	2.1		0	0		1.8						0	1.7
Bicycles	0	0		0		0		0	2	0		2	0		0	0		0						0	2
% Bicycles	0	0		0		0		0	3.2	0		0.4	0		0	0		0						0	0.2
Peds					0	-					3	-					1	-					0	-	4
% Peds					0	-					75	-					25	-					0	-	



### **Peak Hour Diagram**

#### **Specified Period**

#### **One Hour Peak**

From: 15:00:00 To: 18:00:00 From: 16:00:00 To: 17:00:00

**Intersection:** LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

Weather conditions:

Clear

#### \*\* Unsignalized Intersection \*\*

#### Major Road: LAUZON ROAD runs N/S

#### **North Approach**

	Out	In	Total
	374	621	995
MT	1	3	4
HT	2	0	2
<i>₫</i>	1	0	1
	378	624	1002

#### **LAUZON ROAD**

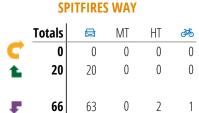
		<b>1</b>	<b>u</b>
Totals	368	10	0
	365	9	0
MT	1	0	0
HT	1	1	0
<i>₫</i>	1	0	0

#### **East Approach**

	Out	In	Total
	83	88	171
MT	0	0	0
ΗТ	2	1	3
<b>₹</b>	1	0	1
	86	89	175

#### Peds: 0





#### Peds: 5

	1		J
Totals	604	79	0
	601	79	0
MT	3	0	0
HT	0	0	0
₫ <b>%</b>	0	0	0

#### **LAUZON ROAD**

#### **South Approach**

	Out	In	Total
	680	428	1108
MT	3	1	4
НТ	0	3	3
<b>ॐ</b>	0	2	2
	683	434	1117



MT - Medium Trucks

HT - Heavy Trucks

♣ - Bicycles

#### **Comments**



### **Peak Hour Summary**

Intersection: LAUZON ROAD & SPITFIRES WAY

 Site Code:
 2207400001

 Count Date:
 Sep 14, 2022

 Period:
 15:00 - 18:00

### **Peak Hour Data (16:00 - 17:00)**

		N	lorth <i>A</i> LAUZO	ipproac N ROAE	:h )			S	outh A LAUZO	pproac N ROAI	:h )				East Ap SPITFIR	proacl RES WA	h Y				West A	pproacl	1		Total Vehicl
Start Time	4	1	•	J	Peds	Total	4	1		•	Peds	Total	4	1	•	•	Peds	Total	4	1	•	•	Peds	Total	es
16:00	2	100		0	0	102		150	14	0	0	164	24		8	0	0	32					0		298
16:15	3	86		0	0	89		150	22	0	0	172	14		6	0	0	20					0		281
16:30	2	89		0	0	91		159	18	0	2	177	16		5	0	0	21					0		289
16:45	3	93		0	0	96		145	25	0	3	170	12		1	0	0	13					0		279
Grand Total	10	368		0	0	378		604	79	0	5	683	66		20	0	0	86					0	0	1147
Approach %	2.6	97.4		0		-		88.4	11.6	0		-	76.7		23.3	0		-						-	
Totals %	0.9	32.1		0		33		52.7	6.9	0		59.5	5.8		1.7	0		7.5						0	
PHF	0.83	0.92		0		0.93		0.95	0.79	0		0.96	0.69		0.63	0		0.67						0	0.96
Cars	9	365		0		374		601	79	0		680	63		20	0		83						0	1137
% Cars	90	99.2		0		98.9		99.5	100	0		99.6	95.5		100	0		96.5						0	99.1
Medium Trucks	0	1		0		1		3	0	0		3	0		0	0		0						0	4
% Medium Trucks	0	0.3		0		0.3		0.5	0	0		0.4	0		0	0		0						0	0.3
Heavy Trucks	1	1		0		2		0	0	0		0	2		0	0		2						0	4
% Heavy Trucks	10	0.3		0		0.5		0	0	0		0	3		0	0		2.3						0	0.3
Bicycles	0	1		0		1		0	0	0		0	1		0	0		1						0	2
% Bicycles	0	0.3		0		0.3		0	0	0		0	1.5		0	0		1.2						0	0.2
Peds					0	-					5	-					0	-					0	-	5
% Peds					0	-					100	-					0	-					0	-	

## Appendix C

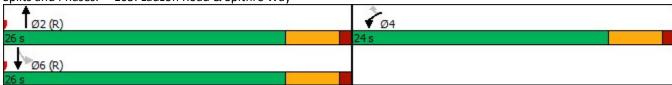
Synchro Analysis Worksheets

	•	•	<b>†</b>	~	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> 1>			414
Traffic Volume (vph)	36	10	271	49	16	375
Future Volume (vph)	36	10	271	49	16	375
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0			U	30.0	
		1.00	0.95	0.95	0.95	0.95
Lane Util. Factor	1.00	1.00		0.95	0.95	
Ped Bike Factor	1.00	0.050	1.00			1.00
Frt	0.050	0.850	0.977			0.000
Flt Protected	0.950		a =			0.998
Satd. Flow (prot)	1722	1484	3427	0	0	3532
Flt Permitted	0.950					0.934
Satd. Flow (perm)	1720	1484	3427	0	0	3306
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		13	51			
Link Speed (k/h)	50		50			50
Link Distance (m)	177.3		508.6			95.7
Travel Time (s)	12.8		36.6			6.9
Confl. Peds. (#/hr)	1			2	2	
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	6%	10%	4%	2%	6%	3%
Adj. Flow (vph)	47	13	352	64	21	487
Shared Lane Traffic (%)	47	13	332	04	21	407
	47	12	116	0	0	F00
Lane Group Flow (vph)	47	13	416	0	0	508
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase	7	7			J	3
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
	19.0	19.0	21.0			21.0
Minimum Split (s)					21.0	
Total Split (s)	24.0	24.0	26.0		26.0	26.0
	48.0%	48.0%	52.0%		52.0%	52.0%
Maximum Green (s)	19.0	19.0	21.0		21.0	21.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Ecoa Lab Optimize:						

22-3402 Synchro 10 Report

	1	*	<b>†</b>	1	-	<b>↓</b>	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0	7.0				
Pedestrian Calls (#/hr)	1	1	2				
Act Effct Green (s)	10.0	10.0	37.2			37.2	
Actuated g/C Ratio	0.20	0.20	0.74			0.74	
v/c Ratio	0.14	0.04	0.16			0.21	
Control Delay	15.7	7.6	4.3			4.9	
Queue Delay	0.0	0.0	0.0			0.0	
Total Delay	15.7	7.6	4.3			4.9	
LOS	В	Α	Α			Α	
Approach Delay	14.0		4.3			4.9	
Approach LOS	В		Α			Α	
Queue Length 50th (m)	3.9	0.0	5.7			8.4	
Queue Length 95th (m)	6.8	2.1	14.6			20.2	
Internal Link Dist (m)	153.3		484.6			71.7	
Turn Bay Length (m)	25.0						
Base Capacity (vph)	654	571	2562			2459	
Starvation Cap Reductn	0	0	0			0	
Spillback Cap Reductn	0	0	0			0	
Storage Cap Reductn	0	0	0			0	
Reduced v/c Ratio	0.07	0.02	0.16			0.21	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length:	50						
Offset: 18 (36%), Referenced to phase 2:NBT and 6:SBTL, Start of Green							
Natural Cycle: 40							
Control Type: Actuated-	Coordinate	ed					
Maximum v/c Ratio: 0.2	.1						
Intersection Signal Delay: 5.2					Intersection LOS: A		
Intersection Capacity Utilization 37.1%					ICU Level of Service A		
A 1 : D : 1/ : \45							

Splits and Phases: 105: Lauzon Road & Spitfire Way

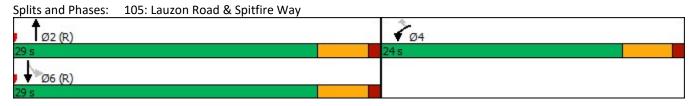


22-3402 Synchro 10 Report

Analysis Period (min) 15

	•	•	<b>†</b>	~	-	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	67	20	610	80	10	372
Future Volume (vph)	67	20	610	80	10	372
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	1300	0.0	0.0	2300
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0	_		U	30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.55	0.55	0.55	0.55
Frt	1.00	0.850	0.983			
Flt Protected	0.950	0.050	0.565			0.999
Satd. Flow (prot)	1772	1633	3588	0	0	3637
Flt Permitted	0.950	1033	3300	U	U	0.939
Satd. Flow (perm)		1633	2500	0	0	
· · · · · · · · · · · · · · · · · · ·	1764		3588		U	3419
Right Turn on Red		Yes	25	Yes		
Satd. Flow (RTOR)		21	35			F.0
Link Speed (k/h)	50		50			50
Link Distance (m)	213.1		512.7			78.2
Travel Time (s)	15.3		36.9			5.6
Confl. Peds. (#/hr)	5	0.55		0.00	0.55	0.55
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	70	21	635	83	10	388
Shared Lane Traffic (%)						
Lane Group Flow (vph)	70	21	718	0	0	398
<b>Enter Blocked Intersection</b>		No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
• •						
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	1	•	<b>†</b>	1	1	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.2	10.2	40.0			40.0
Actuated g/C Ratio	0.19	0.19	0.75			0.75
v/c Ratio	0.21	0.06	0.26			0.15
Control Delay	18.0	7.5	4.6			4.5
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	18.0	7.5	4.6			4.5
LOS	В	Α	Α			Α
Approach Delay	15.6		4.6			4.5
Approach LOS	В		Α			Α
Queue Length 50th (m)	6.3	0.0	11.8			6.3
Queue Length 95th (m)	11.7	3.6	32.8			18.7
Internal Link Dist (m)	189.1		488.7			54.2
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	598	2717			2581
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.11	0.04	0.26			0.15
Intersection Summary						
Area Type:	Other					
Cycle Length: 53						
Actuated Cycle Length: 53	3					
Offset: 20 (38%), Referen	ced to ph	ase 2:NI	3T and 6:5	SBTL, Sta	art of Gr	een
Natural Cycle: 40						
Control Type: Actuated-C	Coordinate	ed				
Maximum v/c Ratio: 0.26						
Intersection Signal Delay:	: 5.4			In	tersecti	on LOS: A
Intersection Capacity Util		1.4%		IC	CU Level	of Service
Analysis Period (min) 15						



	•	•	<b>†</b>	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	67	20	487	80	10	436
Future Volume (vph)	67	20	487	80	10	436
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0	-			30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.33	5.55	0.33	0.33
Frt	1.00	0.850	0.979			
Flt Protected	0.950	0.050	0.575			0.999
Satd. Flow (prot)	1772	1633	3573	0	0	3639
Flt Permitted	0.950	1033	33/3	U	- 0	0.944
Satd. Flow (perm)	1764	1633	3573	0	0	3438
· · · · · · · · · · · · · · · · · · ·	1/04		33/3		U	3438
Right Turn on Red Satd. Flow (RTOR)		Yes	45	Yes		
· · · · · · · · · · · · · · · · · · ·	F.0	21	45			F.0
Link Speed (k/h)	50		50			50
Link Distance (m)	191.6		515.5			65.0
Travel Time (s)	13.8		37.1			4.7
Confl. Peds. (#/hr)	5	0.00	0.00	0.00	0.00	0.00
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	70	21	507	83	10	454
Shared Lane Traffic (%)						
Lane Group Flow (vph)	70	21	590	0	0	464
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	•	•	1	1	1	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.2	10.2	40.0			40.0
Actuated g/C Ratio	0.19	0.19	0.75			0.75
v/c Ratio	0.21	0.06	0.22			0.18
Control Delay	18.0	7.5	4.4			4.6
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	18.0	7.5	4.4			4.6
LOS	В	Α	Α			Α
Approach Delay	15.6		4.4			4.6
Approach LOS	В		Α			Α
Queue Length 50th (m)	6.3	0.0	9.0			7.5
Queue Length 95th (m)	11.7	3.6	25.7			21.8
Internal Link Dist (m)	167.6		491.5			41.0
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	598	2709			2596
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.11	0.04	0.22			0.18
Intersection Summary						
Area Type:	Other					
Cycle Length: 53						
Actuated Cycle Length:	53					
Offset: 20 (38%), Refere	nced to ph	ase 2:NI	BT and 6:	SBTL, St	art of G	reen
Natural Cycle: 40						
Control Type: Actuated-	-Coordinate	ed				
Maximum v/c Ratio: 0.2	2					

Intersection Signal Delay: 5.3

Analysis Period (min) 15

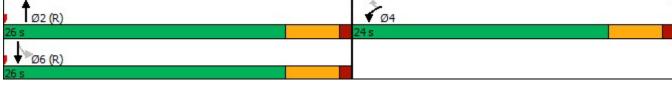
Intersection Capacity Utilization 34.2%

Intersection LOS: A

ICU Level of Service A

	1	•	<b>†</b>	-	1	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		7	<b>†</b> \$			414
Traffic Volume (vph)	106	15	296	77	20	425
Future Volume (vph)	106	15	296	77	20	425
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0	-			30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	1.00	0.55	0.55	1.00
Frt	1.00	0.850	0.969			1.00
	0.950	0.000	0.505			0.998
Satd. Flow (prot)	1722	1484	3398	0	0	3532
	0.950	1707	3330	U	U	0.926
Satd. Flow (perm)	1720	1484	3398	0	0	3277
Right Turn on Red	1/20	Yes	3330	Yes	- 0	3211
Satd. Flow (RTOR)		19	80	163		
Link Speed (k/h)	50	19	50			50
	177.3		508.6			95.7
• ,	177.3					6.9
Travel Time (s) Confl. Peds. (#/hr)	12.8		36.6	2	2	6.9
\ / /		0.77	0.77			0.77
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	6%	10%	4%	2%	6%	3%
Adj. Flow (vph)	138	19	384	100	26	552
Shared Lane Traffic (%)	4					
Lane Group Flow (vph)	138	19	484	0	0	578
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase			_			
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	26.0		26.0	26.0
	48.0%	48.0%	52.0%		52.0%	52.0%
Maximum Green (s)	19.0	19.0	21.0		21.0	21.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0	7.0				
Pedestrian Calls (#/hr)	1	1	2				
Act Effct Green (s)	10.6	10.6	33.0			33.0	
Actuated g/C Ratio	0.21	0.21	0.66			0.66	
v/c Ratio	0.38	0.06	0.21			0.27	
Control Delay	18.8	6.9	4.9			6.0	
Queue Delay	0.0	0.0	0.0			0.0	
Total Delay	18.8	6.9	4.9			6.0	
LOS	В	Α	Α			Α	
Approach Delay	17.4		4.9			6.0	
Approach LOS	В		Α			Α	
Queue Length 50th (m)	11.8	0.0	6.8			10.4	
Queue Length 95th (m)	15.6	2.6	16.2			23.1	
Internal Link Dist (m)	153.3		484.6			71.7	
Turn Bay Length (m)	25.0						
Base Capacity (vph)	654	575	2271			2164	
Starvation Cap Reductn	0	0	0			0	
Spillback Cap Reductn	0	0	0			0	
Storage Cap Reductn	0	0	0			0	
Reduced v/c Ratio	0.21	0.03	0.21			0.27	
Intersection Summary							
Area Type:	Other						
Cycle Length: 50							
Actuated Cycle Length: 50	0						
Offset: 18 (36%), Referen	ced to ph	ase 2:N	3T and 6:	SBTL, Sta	art of Gr	een	
Natural Cycle: 40							
Control Type: Actuated-C	oordinate	ed					
Maximum v/c Ratio: 0.38							
Intersection Signal Delay:	7.1			Ir	ntersecti	on LOS: A	4
Intersection Capacity Util	ization 41	5%		IC	CU Level	of Servic	e A
Analysis Period (min) 15							
Splits and Phases: 105:	Lauzon R	oad & S	pitfire Wa	ау			
<b>+</b>					- 4	3	



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	118	27	668	159	16	410
Future Volume (vph)	118	27	668	159	16	410
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0				30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					
Frt	2.50	0.850	0.971			
Flt Protected	0.950					0.998
Satd. Flow (prot)	1772	1633	3544	0	0	3629
Flt Permitted	0.950		33 17			0.919
Satd. Flow (perm)	1764	1633	3544	0	0	3342
Right Turn on Red	1704	Yes	3344	Yes	U	3342
Satd. Flow (RTOR)		28	72	1 53		
Link Speed (k/h)	50	20	50			50
Link Distance (m)	213.1		512.7			78.2
Travel Time (s)	15.3		36.9			5.6
Confl. Peds. (#/hr)	15.3		30.9			٥.د
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
	3%	0.96	0.96	0.96	10%	0.96
Heavy Vehicles (%)	123		696			
Adj. Flow (vph)	123	28	090	166	17	427
Shared Lane Traffic (%)	422	20	0.00		0	444
Lane Group Flow (vph)	123	28	862	0	0	444
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
	5.0	5.0	5.0			5.0
Lead/Lag Ontimize?						
Lead-Lag Optimize?						

Ø6 (R)

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.8	10.8	35.8			35.8
Actuated g/C Ratio	0.20	0.20	0.68			0.68
v/c Ratio	0.34	0.08	0.36			0.20
Control Delay	19.4	6.8	5.7			5.5
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	19.4	6.8	5.7			5.5
LOS	В	Α	Α			Α
Approach Delay	17.1		5.7			5.5
Approach LOS	В		А			Α
Queue Length 50th (m)	11.1	0.0	15.8			7.9
Queue Length 95th (m)	18.4	4.2	39.4			21.1
Internal Link Dist (m)	189.1		488.7			54.2
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	603	2417			2257
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.19	0.05	0.36			0.20
Intersection Summary						
	Other					
Cycle Length: 53						
Actuated Cycle Length: 53	3					
Offset: 20 (38%), Reference		ase 2:NI	BT and 6:	SBTL. Sta	art of Gr	een
Natural Cycle: 40				,		
Control Type: Actuated-Co	oordinate	ed				
Maximum v/c Ratio: 0.36						
Intersection Signal Delay:	6.8			Ir	ntersecti	on LOS: A
Intersection Capacity Utili		3.5%				of Service
Analysis Period (min) 15						
Splits and Phases: 105:	Lauzon R	oad & S	pitfire Wa	ay		
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> 1>			41
Traffic Volume (vph)	125	27	550	170	17	479
Future Volume (vph)	125	27	550	170	17	479
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	1000	0.0	0.0	1300
Storage Lanes	1	1		0	0.0	
Taper Length (m)	30.0	_		U	30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.33	0.93	0.93	0.93
Frt	1.00	0.850	0.965			
Flt Protected	0.050	0.850	0.965			0.000
	0.950	1622	2522	0	0	0.998
Satd. Flow (prot)	1772	1633	3522	0	0	3630
Flt Permitted	0.950	4655	0555	-	_	0.925
Satd. Flow (perm)	1764	1633	3522	0	0	3365
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		28	101			
Link Speed (k/h)	50		50			50
Link Distance (m)	191.6		515.5			65.0
Travel Time (s)	13.8		37.1			4.7
Confl. Peds. (#/hr)	5					
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	130	28	573	177	18	499
Shared Lane Traffic (%)						
Lane Group Flow (vph)	130	28	750	0	0	517
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0		LCT	0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
, ,	3.0		3.0			3.0
Two way Left Turn Lane	0.00	0.00	0.00	0.00	0.00	0.00
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
	5.0	5.0	5.0			5.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

Ø6 (R)

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.9	10.9	35.7			35.7
Actuated g/C Ratio	0.21	0.21	0.67			0.67
v/c Ratio	0.36	0.08	0.31			0.23
Control Delay	19.6	6.7	5.3			5.7
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	19.6	6.7	5.3			5.7
LOS	В	Α	Α			Α
Approach Delay	17.3		5.3			5.7
Approach LOS	В		Α			Α
Queue Length 50th (m)	11.7	0.0	12.6			9.6
Queue Length 95th (m)	19.4	4.2	31.7			24.5
Internal Link Dist (m)	167.6		491.5			41.0
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	603	2402			2264
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.20	0.05	0.31			0.23
Intersection Summary						
	Other					
Cycle Length: 53						
Actuated Cycle Length: 53						
Offset: 20 (38%), Reference		ase 2·NF	ST and 6.	SBTI Sta	art of Gr	een
Natural Cycle: 40	ca to pii	use 2	31 and 51	35 12, 310		
Control Type: Actuated-Co	oordinate	h				
Maximum v/c Ratio: 0.36	Jorannace	.u				
Intersection Signal Delay:	6.7			In	tarcacti	ion LOS: A
Intersection Capacity Utili		0%				of Service
Analysis Period (min) 15	2011011 40	.570		ic	O LEVE	OI SEI VICE
Allarysis Feriod (Illill) 13						
Splits and Phases: 105: I	Lauzon R	oad & S	pitfire Wa	эу		
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	7	<b>†</b> 1>			44
Traffic Volume (vph)	106	15	309	77	20	467
Future Volume (vph)	106	15	309	77	20	467
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	1300	0.0	0.0	1300
Storage Lanes	23.0	1		0.0	0.0	
_		1		U		
Taper Length (m)	30.0	1.00	0.05	0.05	30.0	0.05
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	0.5=5	1.00			1.00
Frt		0.850	0.970			
Flt Protected	0.950					0.998
Satd. Flow (prot)	1722	1484	3402	0	0	3532
Flt Permitted	0.950					0.927
Satd. Flow (perm)	1720	1484	3402	0	0	3281
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		19	76			
Link Speed (k/h)	50		50			50
Link Distance (m)	177.3		508.6			95.7
Travel Time (s)	12.8		36.6			6.9
Confl. Peds. (#/hr)	1		30.0	2	2	5.5
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	6%	10%	4%	2%	6%	3%
Adj. Flow (vph)	138	19	401	100	26	606
	130	19	401	100	20	000
Shared Lane Traffic (%)	120	10	F04	0	•	622
Lane Group Flow (vph)	138	19	501	0	0	632
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4	_		6	
Detector Phase	4	4	2		6	6
Switch Phase	7	7			J	J
Minimum Initial (s)	8.0	0.0	10.0		10.0	10.0
		8.0	10.0		10.0	
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	26.0		26.0	26.0
Total Split (%)	48.0%	48.0%	52.0%		52.0%	52.0%
Maximum Green (s)	19.0	19.0	21.0		21.0	21.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

Splits and Phases: 105: Lauzon Road & Spitfire Way

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	1	1	2			
Act Effct Green (s)	10.6	10.6	33.0			33.0
Actuated g/C Ratio	0.21	0.21	0.66			0.66
v/c Ratio	0.38	0.06	0.22			0.29
Control Delay	18.8	6.9	5.0			6.2
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	18.8	6.9	5.0			6.2
LOS	В	Α	Α			Α
Approach Delay	17.4		5.0			6.2
Approach LOS	В		Α			Α
Queue Length 50th (m)	11.8	0.0	7.2			11.6
Queue Length 95th (m)	15.6	2.6	16.8			25.2
Internal Link Dist (m)	153.3		484.6			71.7
Turn Bay Length (m)	25.0					
Base Capacity (vph)	654	575	2272			2166
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.21	0.03	0.22			0.29
Intersection Summary						
/ 1	Other					
Cycle Length: 50						
Actuated Cycle Length: 50	)					
Offset: 18 (36%), Reference	ced to ph	ase 2:N	3T and 6:	SBTL, St	art of Gr	een
Natural Cycle: 40						
Control Type: Actuated-C	oordinate	ed				
Maximum v/c Ratio: 0.38						
Intersection Signal Delay:	7.1			Ir	ntersecti	ion LOS: A
Intersection Capacity Utili	ization 42	2.6%		10	CU Level	of Service
Analysis Period (min) 15						

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7	7	<b>†</b> 1>			414
Traffic Volume (vph)	118	27	703	159	16	432
Future Volume (vph)	118	27	703	159	16	432
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0	0	
Taper Length (m)	30.0				30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					
Frt		0.850	0.972			
Flt Protected	0.950					0.998
Satd. Flow (prot)	1772	1633	3548	0	0	3630
Flt Permitted	0.950					0.919
Satd. Flow (perm)	1764	1633	3548	0	0	3342
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		28	67			
Link Speed (k/h)	50		50			50
Link Distance (m)	213.1		512.7			78.2
Travel Time (s)	15.3		36.9			5.6
Confl. Peds. (#/hr)	5		20.0			2.3
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	123	28	732	166	17	450
Shared Lane Traffic (%)				_50	-,	.50
Lane Group Flow (vph)	123	28	898	0	0	467
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0		2010	0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane	3.0		5.0			5.0
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15	0.33	15	25	0.99
Turn Type	Prot	Perm	NA	13	Perm	NA
Protected Phases	4	1 CIIII	2		Term	6
Permitted Phases	4	4			6	0
Detector Phase	4	4	2		6	6
Switch Phase	4	4			Ö	Ö
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
	19.0	19.0	21.0		10.0 21.0	21.0
Minimum Split (s)						
Total Split (s)	24.0	24.0	29.0 54.7%		29.0	29.0 54.7%
	45.3%	45.3%			54.7%	
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	•	•	<b>†</b>	1	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.8	10.8	35.8			35.8
Actuated g/C Ratio	0.20	0.20	0.68			0.68
v/c Ratio	0.34	0.08	0.37			0.21
Control Delay	19.4	6.8	5.9			5.5
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	19.4	6.8	5.9			5.5
LOS	В	Α	Α			Α
Approach Delay	17.1		5.9			5.5
Approach LOS	В		Α			Α
Queue Length 50th (m)	11.1	0.0	16.8			8.5
Queue Length 95th (m)	18.4	4.2	41.9			22.2
Internal Link Dist (m)	189.1		488.7			54.2
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	603	2418			2257
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.19	0.05	0.37			0.21
Intersection Summary						
	Other					
Cycle Length: 53						
Actuated Cycle Length: 5	3					
Offset: 20 (38%), Referen		ase 2:NE	BT and 6:	SBTL, St	art of Gr	een
Natural Cycle: 40	·					
Control Type: Actuated-C	Coordinate	ed				
Maximum v/c Ratio: 0.37						
Intersection Signal Delay				lı	ntersecti	ion LOS: A
Intersection Capacity Util		0.5%		10	CU Level	of Service
Analysis Period (min) 15						



	•	•	<b>†</b>	~	-	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	125	27	579	170	17	507
Future Volume (vph)	125	27	579	170	17	507
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	2500	0.0	0.0	2500
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0			U	30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.33	0.33	0.33	0.93
Frt	1.00	0.850	0.966			
Flt Protected	0.050	0.630	0.900			0.000
	0.950	1622	2526	0	0	0.998 3631
Satd. Flow (prot)	1772	1633	3526	0	0	
Flt Permitted	0.950	1600	2526		-	0.926
Satd. Flow (perm)	1764	1633	3526	0	0	3369
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		28	94			
Link Speed (k/h)	50		50			50
Link Distance (m)	191.6		515.5			65.0
Travel Time (s)	13.8		37.1			4.7
Confl. Peds. (#/hr)	5					
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	130	28	603	177	18	528
Shared Lane Traffic (%)						
Lane Group Flow (vph)	130	28	780	0	0	546
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7	Ment	0.0	Merit	LCT	0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
•	3.0		3.0			3.0
Two way Left Turn Lane	0.00	0.00	0.00	0.00	0.00	0.00
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	•	•	<b>†</b>	~	-	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	10.9	10.9	35.7			35.7
Actuated g/C Ratio	0.21	0.21	0.67			0.67
v/c Ratio	0.36	0.08	0.32			0.24
Control Delay	19.6	6.7	5.4			5.7
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	19.6	6.7	5.4			5.7
LOS	В	Α	Α			Α
Approach Delay	17.3		5.4			5.7
Approach LOS	В		Α			Α
Queue Length 50th (m)	11.7	0.0	13.5			10.3
Queue Length 95th (m)	19.4	4.2	33.8			25.9
Internal Link Dist (m)	167.6		491.5			41.0
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	603	2403			2267
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.20	0.05	0.32			0.24
Intersection Summary						
	Other					
Cycle Length: 53						
Actuated Cycle Length: 5	3					
Offset: 20 (38%), Referen	nced to ph	ase 2:NI	BT and 6:	SBTL, St	art of Gr	een
Natural Cycle: 40						
Control Type: Actuated-C	Coordinate	ed				
Maximum v/c Ratio: 0.36	5					
Intersection Signal Delay:	: 6.8			l:	ntersecti	ion LOS: A
Intersection Capacity Util		6%		10	CU Level	of Service A
Analysis Period (min) 15						
Splits and Phases: 105:	: Lauzon R	oad & S	pitfire Wa	ay		
<b>A</b>					9	4.



	1	•	<b>†</b>	-	1	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	113	18	311	103	27	448
Future Volume (vph)	113	18	311	103	27	448
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0				30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.99	0.55	0.55	1.00
Frt	1.00	0.850	0.963			1.00
Flt Protected	0.950	0.000	0.505			0.997
Satd. Flow (prot)	1722	1484	3376	0	0	3527
Flt Permitted	0.950	1-10-7	3370	- 0	- 0	0.910
Satd. Flow (perm)	1720	1484	3376	0	0	3219
Right Turn on Red	1/20	Yes	3370	Yes	U	3213
Satd. Flow (RTOR)		23	112	162		
Link Speed (k/h)	50	23	50			50
Link Speed (k/n) Link Distance (m)	177.3		508.6			95.7
Travel Time (s)	177.3		36.6			6.9
Confl. Peds. (#/hr)	12.8		30.0	2	2	6.9
( ) /	0.77	0.77	0.77	0.77	0.77	0.77
Peak Hour Factor		0.77	0.77			0.77
Heavy Vehicles (%)	6%	10%	4%	2%	6%	3%
Adj. Flow (vph)	147	23	404	134	35	582
Shared Lane Traffic (%)	4.47	22	F30	_	_	64-
Lane Group Flow (vph)	147	23	538	0	0	617
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	26.0		26.0	26.0
	48.0%	48.0%	52.0%		52.0%	52.0%
Maximum Green (s)	19.0	19.0	21.0		21.0	21.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0	7.0				
Pedestrian Calls (#/hr)	1	1	2				
Act Effct Green (s)	10.7	10.7	32.9			32.9	
Actuated g/C Ratio	0.21	0.21	0.66			0.66	
v/c Ratio	0.40	0.07	0.24			0.29	
Control Delay	19.0	6.5	4.8			6.3	
Queue Delay	0.0	0.0	0.0			0.0	
Total Delay	19.0	6.5	4.8			6.3	
LOS	В	Α	Α			Α	
Approach Delay	17.3		4.8			6.3	
Approach LOS	В		Α			Α	
Queue Length 50th (m)	12.5	0.0	7.4			11.6	
Queue Length 95th (m)	16.4	2.8	16.9			24.7	
Internal Link Dist (m)	153.3		484.6			71.7	
Turn Bay Length (m)	25.0						
Base Capacity (vph)	654	578	2258			2116	
Starvation Cap Reductn	0	0	0			0	
Spillback Cap Reductn	0	0	0			0	
Storage Cap Reductn	0	0	0			0	
Reduced v/c Ratio	0.22	0.04	0.24			0.29	
Intersection Summary							
Area Type: O	ther						
Cycle Length: 50							
Actuated Cycle Length: 50							
Offset: 18 (36%), Referenc		ase 2:NI	BT and 6:	SBTL, St	art of Gr	een	
Natural Cycle: 40	•						
Control Type: Actuated-Co	ordinate	ed					
Maximum v/c Ratio: 0.40							
Intersection Signal Delay:	7.1			I	ntersecti	on LOS:	A
Intersection Capacity Utiliz	zation 44	.3%		ŀ	CU Level	of Service	ce A
Analysis Period (min) 15							
Splits and Phases: 105: L	_auzon R	oad & S	pitfire W	ay			
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	•	•	<b>†</b>	~	-	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> 1>			414
Traffic Volume (vph)	153	37	705	179	22	431
Future Volume (vph)	153	37	705	179	22	431
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0	0	
Taper Length (m)	30.0	_			30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.33	0.93	0.93	0.93
Frt	1.00	0.850	0.970			
	0.050	0.850	0.970			0.000
Flt Protected	0.950	4.022	25.44	0	0	0.998
Satd. Flow (prot)	1772	1633	3541	0	0	3625
Flt Permitted	0.950				_	0.900
Satd. Flow (perm)	1764	1633	3541	0	0	3269
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		39	77			
Link Speed (k/h)	50		50			50
Link Distance (m)	213.1		512.7			78.2
Travel Time (s)	15.3		36.9			5.6
Confl. Peds. (#/hr)	5					
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	159	39	734	186	23	449
Shared Lane Traffic (%)	100	33	, 54	100		. 13
Lane Group Flow (vph)	159	39	920	0	0	472
Enter Blocked Intersectio		No	No	No	No	No
	Left					Left
Lane Alignment		Right	Left	Right	Left	
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	11.4	11.4	35.2			35.2
Actuated g/C Ratio	0.22	0.22	0.66			0.66
v/c Ratio	0.42	0.10	0.39			0.22
Control Delay	20.2	6.1	6.2			5.9
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	20.2	6.1	6.2			5.9
LOS	С	Α	Α			Α
Approach Delay	17.4		6.2			5.9
Approach LOS	В		Α			Α
Queue Length 50th (m)	14.2	0.0	18.6			9.3
Queue Length 95th (m)	23.0	4.9	42.7			22.4
Internal Link Dist (m)	189.1		488.7			54.2
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	610	2375			2168
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.25	0.06	0.39			0.22
Intersection Summary						
	Other					
Cycle Length: 53						
Actuated Cycle Length: 53	3					
Offset: 20 (38%), Referen		ase 2·NF	ST and 6°	SBTI Sta	art of Gr	een
Natural Cycle: 40	cca to pii	use 2	or and on	3B 1 E, 3 to		CCII
Control Type: Actuated-C	oordinate	۰4				
Maximum v/c Ratio: 0.42		.u				
Intersection Signal Delay:				In	tarcacti	ion LOS: A
Intersection Capacity Util		. <b>n</b> %				of Service
Analysis Period (min) 15	12011011 43	7.070		ic	O LEVE	OI SEI VICE
Analysis Feriod (min) 15						
Splits and Phases: 105:	Lauzon R	oad & S	pitfire Wa	ау		
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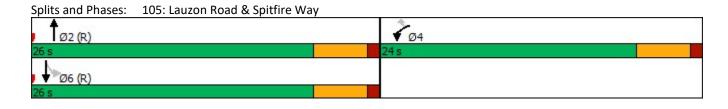
	•	•	†	~	-	ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	165	39	581	192	23	504
Future Volume (vph)	165	39	581	192	23	504
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0	0	
Taper Length (m)	30.0				30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00					
Frt	_,,,,	0.850	0.963			
Flt Protected	0.950		2.300			0.998
Satd. Flow (prot)	1772	1633	3515	0	0	3627
Flt Permitted	0.950		3313			0.910
Satd. Flow (perm)	1764	1633	3515	0	0	3307
Right Turn on Red	1704	Yes	3313	Yes	U	3307
Satd. Flow (RTOR)		41	111	1 53		
Link Speed (k/h)	50	41	50			50
Link Distance (m)	191.6		515.5			65.0
Travel Time (s)	13.8		37.1			4.7
Confl. Peds. (#/hr)	13.8		3/.1			4./
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0.96	0.96	0.96	10%	0.96
, ,						
Adj. Flow (vph)	172	41	605	200	24	525
Shared Lane Traffic (%)	172	44	005	0	^	F 40
Lane Group Flow (vph)	172	41	805 No	0	0	549
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0		1.0	0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
	5.0	5.0	5.0			5.0
Lead/Lag Ontimize2						
Lead-Lag Optimize?						

	1	*	<b>†</b>	1	-	<b>↓</b>	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0	
Recall Mode	None	None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0	7.0				
Flash Dont Walk (s)	11.0	11.0	7.0				
Pedestrian Calls (#/hr)	5	5	0				
Act Effct Green (s)	11.7	11.7	34.9			34.9	
Actuated g/C Ratio	0.22	0.22	0.66			0.66	
v/c Ratio	0.44	0.10	0.34			0.25	
Control Delay	20.5	6.0	5.7			6.1	
Queue Delay	0.0	0.0	0.0			0.0	
Total Delay	20.5	6.0	5.7			6.1	
LOS	С	Α	Α			Α	
Approach Delay	17.7		5.7			6.1	
Approach LOS	В		Α			Α	
Queue Length 50th (m)	15.4	0.0	15.0			11.4	
Queue Length 95th (m)	24.7	5.0	34.3			26.3	
Internal Link Dist (m)	167.6		491.5			41.0	
Turn Bay Length (m)	25.0						
Base Capacity (vph)	635	611	2355			2180	
Starvation Cap Reductn	0	0	0			0	
Spillback Cap Reductn	0	0	0			0	
Storage Cap Reductn	0	0	0			0	
Reduced v/c Ratio	0.27	0.07	0.34			0.25	
Intersection Summary							
Area Type:	Other						
Cycle Length: 53							
Actuated Cycle Length: 53	3						
Offset: 20 (38%), Referen	ced to ph	ase 2:NI	BT and 6:	SBTL, St	art of Gr	een	
Natural Cycle: 40							
Control Type: Actuated-C	Coordinate	ed					
Maximum v/c Ratio: 0.44							
Intersection Signal Delay:	7.5			I	ntersecti	on LOS: A	
Intersection Capacity Util	ization 48	3.3%		ŀ	CU Level	of Service	Α
Analysis Period (min) 15							
Splits and Phases: 105:	Lauzon R	oad & S	pitfire Wa	ay			
<b>A</b>					9	1	



	•	*	<b>†</b>	~	1	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	113	18	324	103	27	490
Future Volume (vph)	113	18	324	103	27	490
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0	_			30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.99	5.55	0.55	1.00
Frt	1.00	0.850	0.964			1.00
Flt Protected	0.950	0.050	0.504			0.997
Satd. Flow (prot)	1722	1484	3380	0	0	3528
Flt Permitted	0.950	1404	3360	U	U	0.912
Satd. Flow (perm)	1720	1484	3380	0	0	3227
Right Turn on Red	1/20	Yes	3360	Yes	U	SZZ/
		yes 23	105	res		
Satd. Flow (RTOR)	F.0	23				F0
Link Speed (k/h)	50		50			50
Link Distance (m)	177.3		508.6			95.7
Travel Time (s)	12.8		36.6	2	2	6.9
Confl. Peds. (#/hr)	1	0.77	0.77	2	2	0.77
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles (%)	6%	10%	4%	2%	6%	3%
Adj. Flow (vph)	147	23	421	134	35	636
Shared Lane Traffic (%)	_					
Lane Group Flow (vph)	147	23	555	0	0	671
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase		<del>-</del>				
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	26.0		26.0	26.0
	48.0%	48.0%	52.0%		52.0%	52.0%
Maximum Green (s)	19.0	19.0	21.0		21.0	21.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	•	•	<b>†</b>	-	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	1	1	2			
Act Effct Green (s)	10.7	10.7	32.9			32.9
Actuated g/C Ratio	0.21	0.21	0.66			0.66
v/c Ratio	0.40	0.07	0.25			0.32
Control Delay	19.0	6.5	4.9			6.4
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	19.0	6.5	4.9			6.4
LOS	В	Α	Α			Α
Approach Delay	17.3		4.9			6.4
Approach LOS	В		Α			Α
Queue Length 50th (m)	12.5	0.0	7.9			12.8
Queue Length 95th (m)	16.4	2.8	17.8			27.2
Internal Link Dist (m)	153.3		484.6			71.7
Turn Bay Length (m)	25.0					
Base Capacity (vph)	654	578	2258			2122
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.22	0.04	0.25			0.32
Intersection Summary						
Area Type:	Other					
Cycle Length: 50						
Actuated Cycle Length: 50	)					
Offset: 18 (36%), Referen	ced to ph	ase 2:Ni	BT and 6:	SBTL, St	art of Gr	reen
Natural Cycle: 40						
Control Type: Actuated-C	oordinate	ed				
Maximum v/c Ratio: 0.40						
Intersection Signal Delay:	7.2			Ir	ntersecti	ion LOS: A
Intersection Capacity Util	ization 45	5.8%		10	CU Level	of Service
Analysis Period (min) 15						



	1	•	<b>†</b>	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> \$			414
Traffic Volume (vph)	153	37	740	179	22	454
Future Volume (vph)	153	37	740	179	22	454
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0	1000	0.0	0.0	1300
Storage Lanes	1	1		0.0	0.0	
Taper Length (m)	30.0	_		U	30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.55	0.55	0.55	0.55
Frt	1.00	0.850	0.971			
Flt Protected	0.950	0.030	0.5/1			0.998
Satd. Flow (prot)	1772	1633	3544	0	0	3626
Flt Permitted	0.950	1033	3344	U	U	0.900
		1622	2544	0	0	
Satd. Flow (perm)	1764	1633	3544	0	0	3270
Right Turn on Red		Yes	70	Yes		
Satd. Flow (RTOR)		39	73			F.0
Link Speed (k/h)	50		50			50
Link Distance (m)	213.1		512.7			78.2
Travel Time (s)	15.3		36.9			5.6
Confl. Peds. (#/hr)	5	0.55		0.00	0.55	0.55
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	159	39	771	186	23	473
Shared Lane Traffic (%)						
Lane Group Flow (vph)	159	39	957	0	0	496
Enter Blocked Intersection		No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane						
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15		15	25	
Turn Type	Prot	Perm	NA		Perm	NA
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase			_			
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
• •						
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						

	1	•	<b>†</b>	1	-	<b>↓</b>
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	11.4	11.4	35.2			35.2
Actuated g/C Ratio	0.22	0.22	0.66			0.66
v/c Ratio	0.42	0.10	0.40			0.23
Control Delay	20.2	6.1	6.4			5.9
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	20.2	6.1	6.4			5.9
LOS	С	Α	Α			Α
Approach Delay	17.4		6.4			5.9
Approach LOS	В		Α			Α
Queue Length 50th (m)	14.2	0.0	19.8			9.8
Queue Length 95th (m)	23.0	4.9	45.3			23.6
Internal Link Dist (m)	189.1		488.7			54.2
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	610	2375			2169
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.25	0.06	0.40			0.23
Intersection Summary						
Area Type:	Other					
Cycle Length: 53						
Actuated Cycle Length: 5	53					
Offset: 20 (38%), Referen		ase 2:NI	BT and 6:	SBTL, St	art of Gr	een
Natural Cycle: 40	•			<u> </u>		
Control Type: Actuated-0	Coordinate	ed				
Maximum v/c Ratio: 0.42						
Intersection Signal Delay				lı	ntersecti	ion LOS: A
Intersection Capacity Uti		5.6%				of Service
Analysis Period (min) 15						
, , , , , ,						

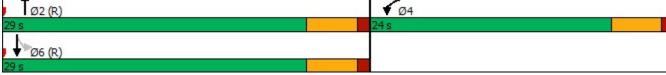
Splits and Phases: 105: Lauzon Road & Spitfire Way



	•	•	<b>†</b>	~	-	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	<b>†</b> 1>			41
Traffic Volume (vph)	165	39	610	192	23	532
Future Volume (vph)	165	39	610	192	23	532
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (m)	25.0	0.0		0.0	0.0	
Storage Lanes	1	1		0	0	
Taper Length (m)	30.0				30.0	
Lane Util. Factor	1.00	1.00	0.95	0.95	0.95	0.95
Ped Bike Factor	1.00	1.00	0.55	0.55	0.55	0.55
Frt	1.00	0.850	0.964			
Flt Protected	0.950	0.050	0.507			0.998
Satd. Flow (prot)	1772	1633	3519	0	0	3628
Flt Permitted	0.950	1033	2213	U	U	0.911
	1764	1633	3519	0	0	3312
Satd. Flow (perm)	1/04		2213		U	3312
Right Turn on Red		Yes	104	Yes		
Satd. Flow (RTOR)		41	104			F-2
Link Speed (k/h)	50		50			50
Link Distance (m)	191.6		515.5			65.0
Travel Time (s)	13.8		37.1			4.7
Confl. Peds. (#/hr)	5					
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	3%	0%	0%	0%	10%	0%
Adj. Flow (vph)	172	41	635	200	24	554
Shared Lane Traffic (%)						
Lane Group Flow (vph)	172	41	835	0	0	578
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(m)	3.7		0.0			0.0
Link Offset(m)	0.0		0.0			0.0
Crosswalk Width(m)	3.0		3.0			3.0
Two way Left Turn Lane	0.0		0.0			0.0
Headway Factor	0.99	0.99	0.99	0.99	0.99	0.99
Turning Speed (k/h)	25	15	0.33	15	25	0.33
	Prot	Perm	NA	13	Perm	NA
Turn Type		reiiii			reiiii	
Protected Phases	4		2			6
Permitted Phases		4			6	
Detector Phase	4	4	2		6	6
Switch Phase						
Minimum Initial (s)	8.0	8.0	10.0		10.0	10.0
Minimum Split (s)	19.0	19.0	21.0		21.0	21.0
Total Split (s)	24.0	24.0	29.0		29.0	29.0
Total Split (%)	45.3%	45.3%	54.7%		54.7%	54.7%
Maximum Green (s)	19.0	19.0	24.0		24.0	24.0
Yellow Time (s)	4.0	4.0	4.0		4.0	4.0
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	5.0	5.0	5.0			5.0
Lead/Lag						
Lead-Lag Optimize?						
Tead tab Optimize:						

	1	*	<b>†</b>	-	1	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Vehicle Extension (s)	4.0	4.0	4.0		4.0	4.0
Recall Mode	None	None	C-Max		C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0			
Flash Dont Walk (s)	11.0	11.0	7.0			
Pedestrian Calls (#/hr)	5	5	0			
Act Effct Green (s)	11.7	11.7	34.9			34.9
Actuated g/C Ratio	0.22	0.22	0.66			0.66
v/c Ratio	0.44	0.10	0.35			0.26
Control Delay	20.5	6.0	5.9			6.2
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	20.5	6.0	5.9			6.2
LOS	С	Α	Α			Α
Approach Delay	17.7		5.9			6.2
Approach LOS	В		А			Α
Queue Length 50th (m)	15.4	0.0	16.0			12.1
Queue Length 95th (m)	24.7	5.0	36.4			27.8
Internal Link Dist (m)	167.6		491.5			41.0
Turn Bay Length (m)	25.0					
Base Capacity (vph)	635	611	2355			2183
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.27	0.07	0.35			0.26
Intersection Summary						
Area Type:	Other					
Cycle Length: 53						
Actuated Cycle Length:	53					
Offset: 20 (38%), Refere	nced to ph	ase 2:NI	BT and 6:	SBTL, Sta	art of Gr	reen
Natural Cycle: 40						
Control Type: Actuated-	-Coordinate	ed				
Maximum v/c Ratio: 0.4	4					
Intersection Signal Dela	y: 7.5			Ir	ntersecti	ion LOS: A
Intersection Capacity Ut		9.1%		IC	CU Level	of Service





Analysis Period (min) 15

## Appendix D

LOS Definitions

## LEVEL OF SERVICE1

Level of Service (LOS) is defined as a qualitative measure describing operational conditions within a traffic stream, and their perception by motorists and/or passengers. This concept was introduced in the 1965 *Highway Capacity Manual* as a criteria for interrupted flow conditions. The 2000 *Highway Capacity Manual* changed the basis for measuring Level of Service at intersections to control delay<sup>2</sup>.

Six Levels of Service are defined with LOS A representing the best operating conditions, and LOS F the worst (briefly described below). It should be noted that there is often significant variability in the amount of delay experienced by individual drivers.

- LOS A: This Level of Service describes the highest quality of traffic flow and is referred to as free flow. The approach appears open, turning movements are easily made and drivers have freedom of operation. Control delay is less than 10 seconds/vehicle.
- LOS B: This Level of Service is referred to as a stable flow. Drivers feel somewhat restricted and occasionally may have to wait to complete the minor movement. Control delay is 10-15 seconds/vehicle for unsignalized intersections and 10-20 seconds/vehicle for signalized intersections.
- LOS C: At this level, the operation is stable. Drivers feel more restricted and may have to wait, with queues developing for short periods. Control delay is 15-25 seconds/vehicle at unsignalized intersections and 20-35 seconds/vehicle at signalized intersections.
- LOS D: At this level, traffic is approaching unstable flow. The motorist experiences increasing restriction and instability of flow. There are substantial delays to approaching vehicles during short peaks within the peak period, but there are enough gaps to lower demand to permit occasional clearance of developing queues and prevent excessive back-ups. Control delay is 25-35 seconds/vehicle at unsignalized intersections and 35-55 seconds/vehicle at signalized intersections.
- LOS E: At this level capacity occurs. Long queues of vehicles exist and delays to vehicles may extend. Control delay is 35-50 seconds/vehicle at unsignalized intersections and 55-80 seconds/vehicle at signalized intersections.
- LOS F: At this Level of Service, the intersection has failed. Capacity of the intersection has been exceeded. Control delay exceeds 50 seconds/vehicle at unsignalized intersections and exceeds 80 seconds/vehicle at signalized intersections.

<sup>&</sup>lt;sup>1</sup> Transportation Research Board: Highway Capacity Manual 1965, 2000

<sup>&</sup>lt;sup>2</sup> Control delay is defined as the component of delay that results when a control signal causes a lane group to reduce speed or to stop; it is measured by comparison with the uncontrolled condition.